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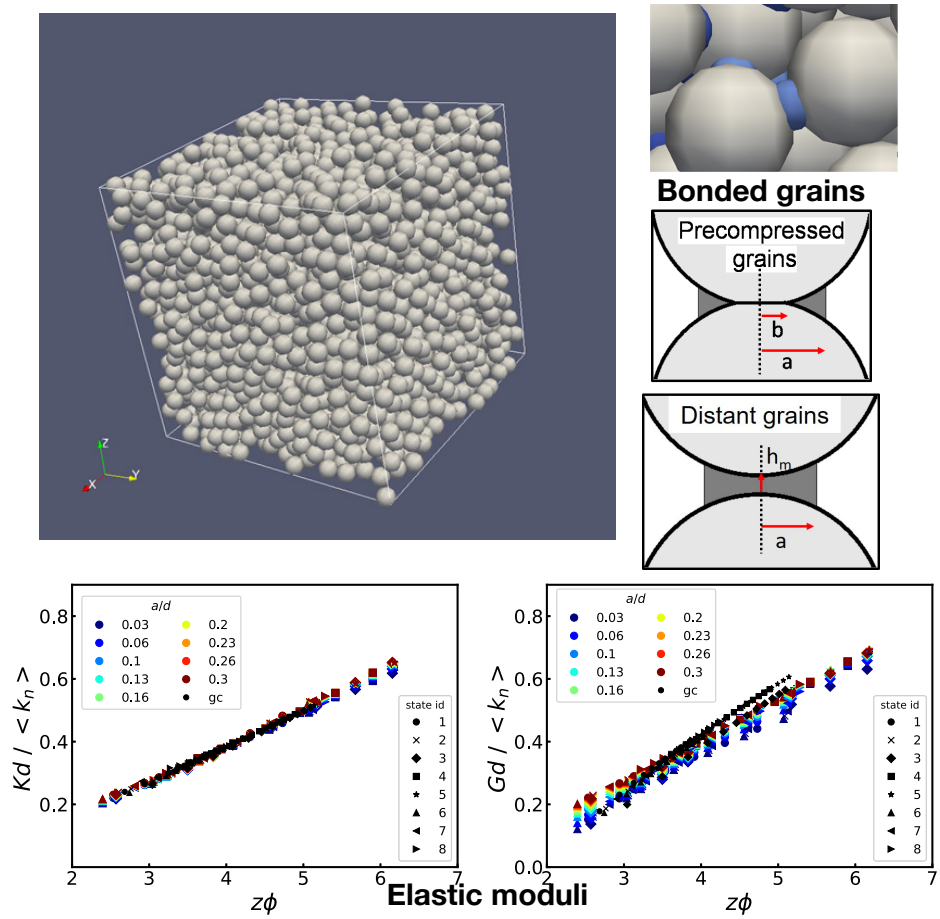
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Graphical Abstract

Elasticity of model weakly cemented granular materials: a numerical study.

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Highlights

Elasticity of model weakly cemented granular materials: a numerical study.

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- Geometry-dependent stiffnesses of cement bonds between grains are computed.
- Bond stiffnesses are used to obtain macroscopic moduli of DEM-simulated grain packs.
- A wide range of mechanically attainable prestressed initial structures is used.
- Resulting moduli prove sensitive to saturation-dependent bond coordination number.
- Effects of angular stiffnesses are assessed and estimation schemes are tested.

Elasticity of model weakly cemented granular materials: a numerical study.

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Abstract

The effect of bonding cementation on the elastic properties of model granular materials is investigated by grain-level numerical simulations. Two different morphologies are studied, bridging cementation, for which the cement concentrates in cylindrical connections, with radius a , between contacting or neighboring grains separated by a distance smaller than a certain range h_0 ; and grain coating, for which a cement layer with uniform thickness Δ forms on grain surfaces. Cementation is applied to spherical ball packs assembled by the discrete element method (DEM) in different isotropic states throughout the range of mechanically accessible structures, with different densities and contact connectivities. The determination of the elastic properties of the cemented material is a two stage procedure, in which (i) bond stiffnesses, and (ii) macroscopic moduli of large bond networks, are successively computed. Our treatment of stage (i) is based on a previously published model (V. Langlois, *International Journal of Numerical and Analytical Methods in Geomechanics*, **39**(8), p. 854, 2015 [1]), suitably generalized to deal with prestressed granular materials in which contacts carry a certain force prior to cementation, and with distant bonding. In stage (ii), different initial packs are compared, and the influence of the model parameters (a , h_0 , Δ) is discussed. The angular elasticity of cemented bonds, for bridging cementation, may influence the macroscopic moduli for small bond coordination number and large enough bond radii, but does not affect grain-coated cemented materials. Macroscopic moduli are essentially determined by the average bond stiffness and by the bond network coordination number. Large confining

stresses prior to cementation tend to stiffen the cemented material. The limit of stiff cement is shown to apply in a restricted range. As for unbonded grains, the Voigt approximation scheme for elastic moduli gives fair predictions for the bulk modulus, but rather poor ones for the shear modulus, in weakly coordinated systems. The scheme advocated by Dvorkin and Nur (*Geophysics*, **61**(5), p. 1363, 1996 [2]), often used in geomechanics, relies on the Voigt approximation. Though yielding correct orders of magnitude, it can be inaccurate, as it does not account for the cement content dependence of coordination numbers.

Keywords: Cemented granular materials, Elasticity, Discrete Element Method, Geomechanics

1. Context and motivations

The mechanical properties of granular materials [3], subject to active research over the past decades [4], are of course crucially dependent on the behavior of intergranular contact regions, where stresses and strains concentrate. In turn, contact mechanics [5] is naturally affected by surface properties, in which microscale geometry and material properties combine to produce specific features such as solid friction. Contact properties are quite sensitive to aging and/or physico-chemical effects, either taking place within the grain material or on grain surfaces, where additional material might accumulate over time.

Different processes might eventually transform the geometry and mechanics of contacts, such that intergranular junctions become larger and stronger, effectively leading to the formation of *cemented granular materials*. Once cemented, grain assemblies become stiffer and stronger solids – thus, over geological time scales, assemblies of sand grains may gradually turn into sandstones. In the case of *sintering*, of ceramics [6, 7] or metal powders [8, 9], the grain material itself is displaced and accumulates near contacts, transforming thin junctions into thicker “necks”, while the sample shrinks. This process is usually carried out on increasing the temperature, and applying large stresses, which favors creep. In engineering practice one may directly inject cement into granular soils to improve their resistance, or exploit the capacity of micro-organisms, in the presence of the appropriate reactants, to induce precipitation of solid materials (e.g., calcium carbonate) in the pores [10, 11]. A similar cementation phenomenon may also occur as gas

hydrates form inside submarine sediments [12]. Methane hydrates being a potential source of energy, this latter phenomenon elicited significant research efforts. Thus Dvorkin *et al.* [13, 14, 2, 15] proposed models for local (at the scale of a grain pair joined by a cement bond) and global (at the material scale) elastic properties of model cemented grains, directly applicable to packs of spherical beads. This approach was adapted and exploited in the interpretation of field measurements [16], in which elastic wave velocities are used to detect the presence of hydrates within submarine sediments.

Numerical simulations of the discrete element method (DEM) type [17] have been greatly contributing to the progress made in understanding and modeling granular materials over the last decades. Some numerical studies addressed, in particular, the elastic properties of model *unbonded* granular materials, for different internal states of granular packs [18]. Although the strain range of quasielastic response is very small [19, 20, 21], such numerical studies revealed how elastic moduli may provide information on internal material variables such as coordination number [21, 22] or contact network anisotropy [23, 24]. Those works also enabled comparisons with laboratory results obtained, in particular, with spherical bead packs [25, 26, 27, 28, 29], and tests of micromechanics-based theoretical predictions [30, 31] of macroscopic moduli. On the other hand, quite a few numerical studies, by DEM [32, 33, 7, 34, 35, 36, 37], or coupling DEM to other numerical methods [38] have also been dealing with *bonded or cemented* granular materials. However, while adopting various forms of contact laws incorporating cement resistance, most of them did not attempt accurate models of elasticity. Those works are indeed mostly motivated by the study of material failure, due to bond rupture. DEM results for the elastic properties of cemented bead packs, amenable to quantitative confrontations with experiments on such model systems [39] and apt to test the predictions of Refs. [2, 15] for macroscopic moduli, are still lacking.

The present paper aims at gathering a set of numerical results for the macroscopic elastic properties of cemented assemblies of spherical balls, prepared by DEM in a wide range of initial states. The amount of cement introduced near intergranular contacts varies, for two different simple morphological assumptions: cylindrical bridges joining particles over gaps thinner than a threshold distance, and grain coating. Keeping the usual framework of granular material micromechanics, i.e., assuming the grains, except in small regions near the contact or connections, behave like rigid solid bodies, we proceed in two steps. First, the behavior of one intergranular cemented bond

is modeled, based on the approaches introduced by Dvorkin et al. [13, 14, 40], revisited by one of us [39, 1]; then, starting from different DEM-assembled isotropically confined bead packs [18, 41], morphological assumptions are used for the distribution of cemented bonds, and the elastic moduli of the cemented material are computed. Those properties differ from their counterparts in unbonded granular materials in two respects: the elastic behaviour of each cemented contact or bond, on the one hand; and the distribution of bonds, on the other hand, since initially noncontacting grains may be joined by a cement bridge.

The local model at the scale of a cemented contact or bond is dealt with in Section 2. The results on macroscopic elastic moduli of the cemented granular material, depending on cement quantity and morphological assumptions, with samples initially prepared by DEM in different states, are presented in Sec. 3. Sec. 4 confronts theoretical predictions to numerical results. The paper ends with a summary and discussion, Sec. 5.

2. Stiffness of cemented intergranular bonds

2.1. Morphologies

We first present the model used for the elasticity of intergranular bonds. Intergranular cementation is assumed to take place in previously assembled spherical grain packs under a certain isotropic pressure P , in one of the forms depicted in Fig. 1: *bridging cementation* (Fig. 1a) or *grain coating* (Fig. 1b). The bridging cementation model introduces two parameters (Fig. 2): radius

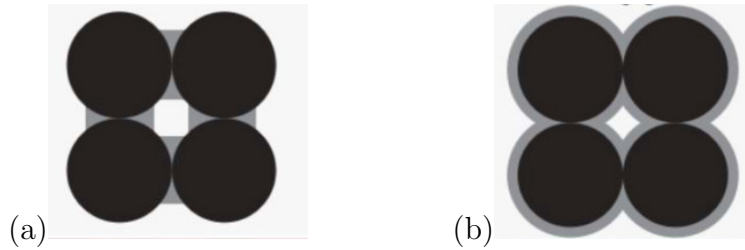


Figure 1: Sketch of model cement morphologies used in our simulations: (a) bridging of grains by formation of cylindrical connections at contacts and in narrow intergranular gaps; (b) grain coating.

a of the cement cylinder, and maximum gap thickness h_0 to be bridged by a cement bond. In the grain coating model, the only parameter is the coating

layer thickness Δ . In both cases, since the cement reinforces a prestressed granular material, some cement bonds connect grains that were already in contact through a finite region, with the shape of a disk of radius b , as shown in Fig. 2a. For the local geometries sketched in Fig. 2, we have to express

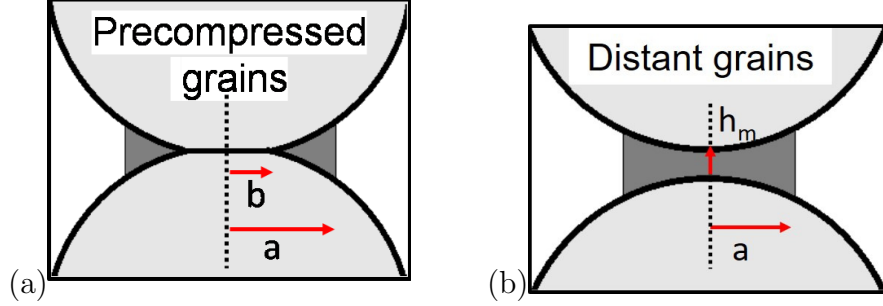


Figure 2: Cemented precompressed contact (a) and cement bond between distant grains (b), separated by gap h_m .

the bond elastic properties as functions of cement cylinder radius a , initial intergranular distance h_m (with $0 \leq h_m \leq h_0$) or initial contact region radius b , and of the elastic properties of grain and cement materials.

2.2. Basic assumptions and definitions

In order to use a mechanical bond model as described below in a bonded granular assembly it has first to be assumed that the grains behave like rigid bodies away from the regions, close enough to the bonded parts of their surfaces, within which strains concentrate. This is referred to as assumption (i) in the following. It is quite usual in contact mechanics [5], in which infinite half-space Green functions are used, and enables models of a system of bonded grains with a set of discrete degrees of freedom (the displacements and the rotations of the grains) in which elastic forces are pairwise additive.

Both grain and cement materials are assumed to be linear, elastic and isotropic. Stress fields are to be determined as solutions to linear integral equations, as written in Refs. [13, 14, 40, 1]. Thanks to the linearity, problems can be solved independently for different relative motions of the grains away from the contact and their solutions superimposed in the general case of an arbitrarily loaded bond.

Specifically, we need expressions of normal stiffness k_n , of tangential stiffness k_t , and of rolling (k_r) and pivoting (k_p) stiffnesses, for a cement bond

joining two spherical beads of radius R , labelled with indices i and j . We first briefly recall here their definitions.

Defining \mathbf{n}_{ij} as the normal unit vector, stiffness k_n relates the normal force increase ΔF_{ij}^N to the normal relative displacement δu_{ij}^N as

$$\Delta F_{ij}^N = k_n^{ij} \delta u_{ij}^N. \quad (1)$$

Similarly, k_t relates the tangential force increase $\Delta \mathbf{F}_{ij}^T$ to the tangential relative displacement $\delta \mathbf{u}_{ij}^T$ of the grain surfaces ($\delta \mathbf{u}_{ij}^T$ involves grain center displacements as well as small rotations $\boldsymbol{\theta}_i$ and $\boldsymbol{\theta}_j$):

$$\Delta \mathbf{F}_{ij}^T = k_t^{ij} \delta \mathbf{u}_{ij}^T, \quad (2)$$

A *moment* is exerted on each grain, as a result of the stress distribution on its surface in contact with the bond. (This moment is evaluated at the center of the contact region, and should not be confused with the moment of the tangential force *at the grain center*). It is related to the relative rotation $\Delta \boldsymbol{\theta}_{ij} = \boldsymbol{\theta}_i - \boldsymbol{\theta}_j$ of the grains, from which the *rolling* (or tangential) component $\Delta \boldsymbol{\theta}_{ij}^{\mathbf{R}}$ is distinguished from the *pivoting* one $\Delta \theta_{ij}^P \mathbf{n}_{ij}$. The bond moment $\boldsymbol{\Gamma}_{ij}$, exerted on grains i and j (with opposite signs) at the center of the bonded surface, in view of the axial symmetry of the bonds, has a rolling (tangential) component $\boldsymbol{\Gamma}_{ij}^{\mathbf{R}}$ and a pivoting one $\Gamma_{ij}^P \mathbf{n}_{ij}$ related to the relative rotation as

$$\begin{aligned} \boldsymbol{\Gamma}_{ij}^{\mathbf{R}} &= k_R^{ij} \Delta \boldsymbol{\theta}_{ij}^{\mathbf{R}} \\ \Gamma_{ij}^P &= k_P^{ij} \Delta \theta_{ij}^P. \end{aligned} \quad (3)$$

Models designed for the elasticity of partially sintered grains, in which initial contact regions have widened and formed “necks” connecting grains, rely on approaches quite similar to the ones pursued here. Interestingly, some corrections to assumption (i) have been proposed in order to account for the interaction between different connections of the same grain [42]. In simulations of sintered grain packs [7], such corrections are roughly accounted for on multiplying each bond stiffness by a constant correction factor. We did not try to implement such features in our simulations, as the study. At least, our approach should be fully justified for small enough cement content¹.

¹On adopting, rather arbitrarily, the same correction factor as in Ref. [7] the obtained correction to bond normal stiffnesses grows with ratio $\alpha = a/R$ of bond radius to grain radius. It is about 10% for $\alpha = 0.2$, decreasing to a few percent for the thinnest bonds, and increasing to more than 50% for the thickest ones, for which our model likely becomes quantitatively inadequate.

We now give our results for the bond stiffnesses, which rely on a second modelling assumption, motivated by the aspect ratio of the cement volume (Fig. 2): the cement bond behaves like a thin slab, with only one non-vanishing strain component [assumption (ii)].

2.3. Normal stiffness

2.3.1. Dimensionless parameters

The normal response (traction/compression), in view of assumption (ii), only involves the longitudinal (or oedometric) modulus M_c of the cement and the normal contact modulus \tilde{E}_g of the grain material. M_c and \tilde{E}_g related to Young moduli E_c , E_g or shear moduli G_c , G_g of the cement (subscript c), or the grain (subscript g) materials, and to their Poisson ratios ν_c , ν_g as

$$\begin{aligned} M_c &= \frac{1 - \nu_c}{(1 + \nu_c)(1 - 2\nu_c)} E_c = \frac{2(1 - \nu_c)}{1 - 2\nu_c} G_c \\ \tilde{E}_g &= \frac{E_g}{1 - \nu_g^2} = \frac{2G_g}{1 - \nu_g}. \end{aligned} \quad (4)$$

Consequently, any result of the compression problem in dimensionless form, such as a normalized stiffness $k_n/(\tilde{E}_g R)$, will only depend on a ratio of those relevant moduli, which, following the literature [14, 2, 1], we define as

$$\Lambda_n = \frac{(1 - \nu_g)M_c}{\pi G_g} = \frac{2M_c}{\pi \tilde{E}_g} \quad (5)$$

The solutions to the normal contact problem when the bridging cementation takes place in a pair of grains initially touching by a single point (i.e., bridging cementation occurring with $b = 0$ in Fig. 2a or $h_m = 0$ in Fig. 2b) are numerically obtained and parametrized in Ref. [2], depending on dimensionless parameters Λ_n and $\alpha = a/R$. Numerical methods to obtain solutions in the cases of precompressed contacts (Fig. 2a) and distant grain bonding (Fig. 2b) were described by Dvorkin *et al.* [14, 40], but those papers do not supply a general functional form fitted to the solution, depending on the three problem parameters (Λ_n , α , and b/R or $h_m R/a^2$). One essential contribution of Ref. [1], which deals with grains initially contacting by a single point, is to show that only one parameter matters in this case, combining Λ_n and α :

$$\beta_n = \frac{\alpha}{\Lambda_n}. \quad (6)$$

Our treatment of precompressed contacts or distant bonding generalizes this approach to deal with precompressed or initially noncontacting grain pairs, and gives bond stiffnesses as functions of two parameters: β_n and b/a , or β_n and $h_m R/a^2$.

2.3.2. The rigid punch limit

An interesting limit is obtained when the cement transmits the stress with only negligible strain, which is in particular achieved for large Λ_n , but more appropriately defined as the limit of small β_n [1]. The normal stiffness is then given by

$$k_n^R = \tilde{E}_g a. \quad (7)$$

This is the *rigid punch limit*, for which the stiffness no longer depends on the cement moduli, and the cement bridge acts as a rigid layer tied to parts of the grain surfaces. Remarkably, the rigid punch limit stiffness of Eq. 7 coincides with the (incremental) stiffness of the preloaded Hertzian contact between non-cemented beads [5], if the normal force is such that the radius of the disk-shaped contact region is a . In other words, k_n takes the value k_n^R in the precompressed contact case of Fig. 2a, when the cement annulus vanishes, $b = a$.

Definition (6) reflects the influences of both material stiffness and geometry. The rigid punch limit may be approached even if the cement is the softer material: because of the geometric factor $\alpha = a/R$, it may still be harder to deform the thin cement slab than the contact regions within the grains.

2.3.3. Numerical solution and parametrization

The solution found by Langlois [1] (after Dvorkin and Nur [2]) may be expressed as a β_n -dependent correction to k_n^R .

The present work gives further parametrizations as proper fits to the numerical solutions of the suitable integral equations for both cases of precompressed and distant cemented grains, which can be readily implemented in a numerical grain pack prepared by DEM. The solutions are now functions of two dimensionless parameters, because β_n is supplemented by b/a in the precompressed case, and by $h_m R/a^2$ for cemented distant grains. The explicit form of these functions is given in Appendix A, and their variations are shown in Fig. 3, over interval $0 \leq \beta_n \leq 10$, for different values of these additional parameters. Note that, on both graphs, the dotted lines, corresponding to $b = 0$ in the precompressed case, and to $h_m = 0$ in the distant

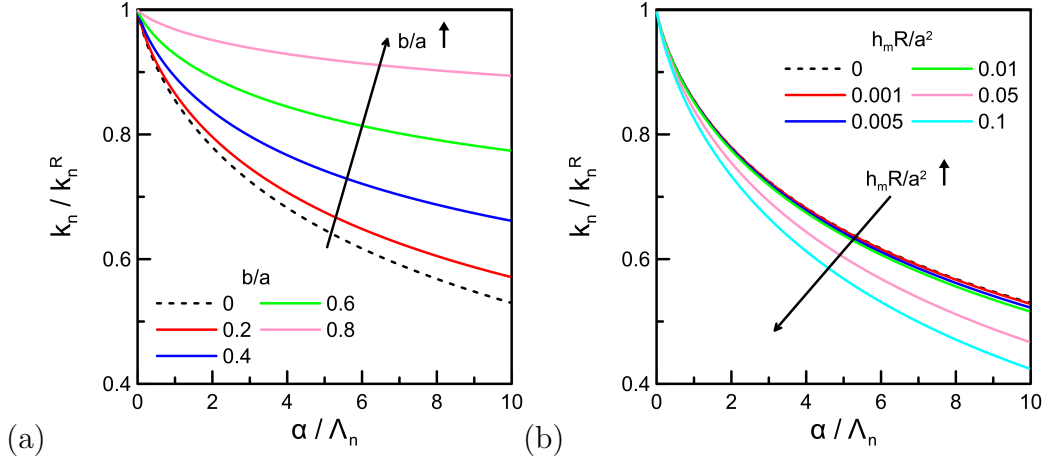


Figure 3: Ratio k_n/k_n^R versus $\beta_n = \alpha/\Lambda_n$ in (a) precompressed contacts, for different values of b/a and (b) cement bridges joining initially distant grains, for different values of $h_m R/a^2$.

bonded grains case, coincide with the result of Ref. [1]. The computed stiffness gets closer to the rigid punch limit as b/a grows (for an increasing force pressing the grains against each other), with limit $k_n/k_n^R = 1$ as $b/a \rightarrow 1$. On the other hand k_n/k_n^R decreases for wider gaps h_m .

2.4. Tangential stiffness

The tangential response of the cement bond, given assumption (ii), only involves the shear modulus G_c of the cement, and the modulus of the grain material involved in the tangential contact law [5], which we denote as E_g^* ,

$$E_g^* = \frac{2 - 2\nu_g}{2 - \nu_g} \tilde{E}_g = \frac{2G_g}{1 - \nu_g/2}. \quad (8)$$

Results of the tangential loading problem of the bond, once in dimensionless form, such as a normalized stiffness $k_t/(E_g^* R)$, will only depend on ratio G_c/E_g^* . After Dvorkin *et al.* [14, 2] and Langlois [1], we introduce

$$\Lambda_t = \frac{G_c}{\pi G_g} = \frac{2G_c}{\pi(1 - \nu_g/2)E_g^*}, \quad (9)$$

bearing in mind that the relevant dimensionless stiffness ratio should rather be defined as $G_c/E_g^* = \frac{\pi}{2}\Lambda_t(1 - \nu_g/2)$. Like in the normal case, the solution

to the tangentially loaded contact problem when the bridging cementation takes place in a pair of grains initially touching by a single point is given in Refs. [13, 14], depending on $\Lambda_t(1 - \nu_g/2)$ and on $\alpha = a/R$, while Ref. [1] expressed it with the sole combined parameter:

$$\beta_t = \frac{\alpha}{(1 - \nu_g/2)\Lambda_t}. \quad (10)$$

The limit of small β_t is the rigid punch limit [1], for which the tangential stiffness is given by

$$k_t^R = E_g^* a. \quad (11)$$

In this limit the bond stiffness no longer depends on cement elasticity and coincides with the Hertz-Mindlin incremental stiffness of a preloaded direct intergranular contact, in which the contact surface is a disk of radius a [5]. Just like in the normal case, we supply in Appendix A parametrized forms of k_t/k_t^R , a function of b/a and β_t for precompressed bonds (Fig. 2a), and a function of $h_m R/a^2$ and β_t for bonds bridging initially noncontacting grains (Fig. 2b).

Models for sintered beads, as implemented, e.g., in Ref. [7] do not involve any cement material, as the bonds are formed as the intergranular contact regions thicken. Consequently, they use the rigid punch formulae (7) and (11) – with a correction due to the interaction between wide enough bonds on the same grain.

2.5. Angular stiffnesses

No accurate computation of the rolling and pivoting stiffnesses, k_r and k_p , was carried out in the present study. Some orders of magnitude might however be expected. As torques might only be transmitted at connections points thanks to the finite lateral extent of the bond, those angular stiffnesses should scale like the linear stiffnesses multiplied by a^2 . One may expect $k_r \propto k_n a^2$, and $k_p \propto k_t a^2$, as normal and tangential displacement fields in the contact region are respectively involved. Dvorkin and Yin [40] write the integral equations the solution of which could be used to compute k_p . In the rigid punch limit, when the strains in the cement bond can be neglected, k_p is given in Ref. [5] as

$$k_p^R = \frac{8}{3} G_g a^3 \quad (12)$$

We do not have results for k_R , and no explicit parametrized form is available for k_P , save for expression (12) in the rigid limit.

The relative importance of terms involving k_R and k_P in the stiffness matrix is expressed by ratios $k_R/(k_n R^2)$, which scale as $(a/R)^2$. Consequently, angular elasticity is likely to have a negligible influence on macroscopic moduli, except perhaps for large enough radius a .

For simplicity we introduce a constant c_0 and assume

$$\begin{aligned} k_r &= c_0 a^2 k_n \\ k_p &= c_0 a^2 k_t. \end{aligned} \tag{13}$$

Note that (12) implies $c_0 = (4 - 2\nu)/3$ ($\simeq 1.13$ for $\nu = 0.3$) in the rigid punch limit.

The influence of c_0 on the macroscopic moduli of the bonded material is assessed in Sec. 3.

2.6. Grain coating morphology

The grain coating morphological model entails connections to appear between all grain pairs separated by a gap h_m lower than 2Δ , with an axisymmetric profile of minimum radius a , given by

$$a = \sqrt{(2\Delta - h_m)R + \Delta^2 - \frac{h_m^2}{4}}. \tag{14}$$

For simplicity, we assume that the stiffnesses of this bond are equal to those of a cylindrical bridge with length h_m and radius a . All contacts ($h_m = 0$) are now reinforced with cylindrical bonds of radius

$$a_0 = \sqrt{2\Delta R + \Delta^2}, \tag{15}$$

while connections between grains at distance h_m growing from 0 to 2Δ decrease in radius according to Eq. 14.

3. Macroscopic moduli of cemented material

3.1. Initial states

In agreement with the description introduced in Sec. 1, we assume the cementation process to take place inside grain packs that were first assembled and compressed without cement. As the elastic properties of granular materials are sensitive to their history-dependent initial structure, we chose a set of quite different initial states, representative of possible granular structures.

The material consists in identical spherical beads, of diameter $d = 2R$. They interact at their contacts by Hertz-Mindlin elasticity [5], and Coulomb friction. They are assembled, as described in Ref. [18], in periodic, homogeneous and isotropic samples, under an isotropic pressure P . All results are averaged over 5 statistically equivalent samples of 4000 spherical beads, for each investigated state. The periodic boundary conditions [17] suppress all wall

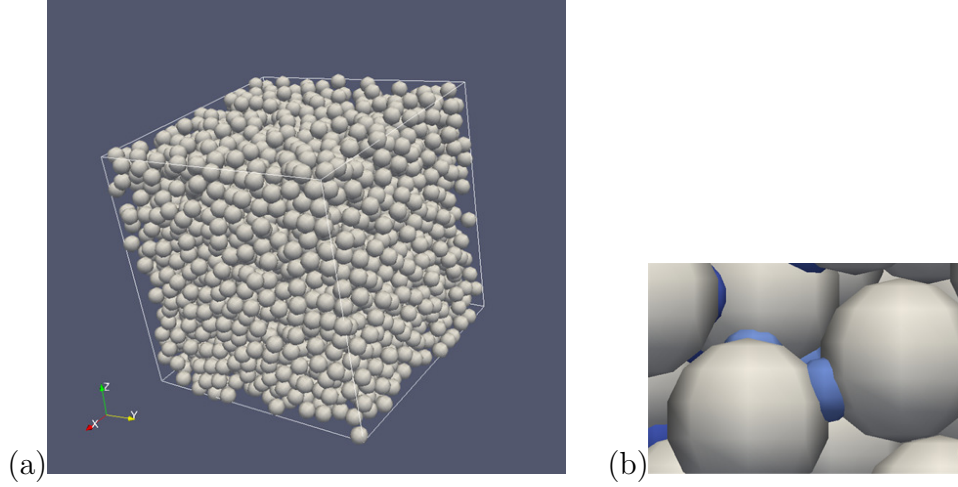


Figure 4: (a) Aspect of a typical simulated spherical grain pack; (b) detail of cement bridges between contacting or neighboring grains.

effects and samples of 4000 grain are quite representative of the macroscopic material, with, as previously observed [21, 24], relative sample to sample fluctuations in elastic properties reaching, at most, a few times 10^{-2} . A typical DEM sample is shown in Figure 4, with the detail of bonded particles (Figure 4b).

The same four initial states defined in Ref. [18] are used here, for intergranular friction coefficient $\mu = 0.3$, and, prior to cementation, they are quasistatically compressed to a pressure P , as in Ref. [43]. We thus build cemented packs from plausible microstructures, corresponding to equilibrated materials under some confining stress. Beads are attributed the elastic properties of glass: Young modulus $E_g = 70$ GPa, Poisson ratio $\nu_g = 0.3$ (and thus shear modulus $G_g = 26.9$ GPa). However, all dimensionless properties

of the material will only depend on the stiffness number, κ , defined as [18]:

$$\kappa = \left(\frac{\tilde{E}_g}{P} \right)^{2/3}, \quad (16)$$

with notation $\tilde{E}_g = E_g/(1 - \nu_g^2) = 2G_g/(1 - \nu_g)$ for the modulus governing contact elasticity. (ν_g determines the ratio of tangential to normal contact stiffnesses, but its influence is observed to be very small [45]). A 5th, looser isotropic state is obtained as in Ref. [41], via a different method involving transitory capillary forces simulating wet beads in a first stage.

Depending on the preparation protocol, those packs vary in their solid fraction Φ , their coordination number z – the average number of force-carrying contacts per grain – and the fraction x_0 of *rattlers*, grains which do not bear any load. We also define the coordination number on the force-carrying network, as the rattler-corrected value:

$$z^* = \frac{z}{1 - x_0}. \quad (17)$$

Those grain packs are isotropically compressed to a pressure near $P = 100$ kPa, corresponding to $\kappa \simeq 8400$ (see Table 1). This value is regarded as typical for some laboratory tests, and within a range over which basic mechanical properties such as internal friction and dilatancy do not significantly vary with confining stress intensity [44, 45], despite some moderate increase of coordination number with pressure [43]. The resulting states, into which bridging cementation is to be introduced or cement grain coating to be applied, are characterized in Table 1. As in previous studies of non-bonded

Table 1: Properties of different initial states of DEM-assembled spherical grain packs under reference isotropic pressure (80 or 100 kPa for glass beads), prior to cementation: solid fraction Φ , rattler-corrected coordination number z^* , rattler fraction x_0 .

State	κ	P (kPa)	Φ	z^*	x_0 (%)
1 (L)	9742	80	0.577 ± 0.002	4.75 ± 0.03	7.4 ± 1.3
2	8395	100	0.594 ± 0.001	4.79 ± 0.03	8.6 ± 0.5
3	8395	100	0.635 ± 0.001	4.82 ± 0.02	12.3 ± 1.9
4	8395	100	0.627 ± 0.001	5.86 ± 0.01	1.4 ± 0.4
5	8395	100	0.638 ± 0.001	6.17 ± 0.005	1.11 ± 0.002

granular packs [18, 43], this set of different states is introduced in order to

explore the variability of possible initial material microstructures. Note, in particular, that the coordination number is not determined by the solid fraction (compare the values of z and Φ in states 3, 4 and 5). The difference between z and z^* , from (17), may exceed 10% and should not be ignored.

In addition, three other states were used (see Tab. 2), differing by the value of pressure P at which cementation will take place. Those states (la-

Table 2: Properties of DEM-assembled spherical grain packs, under different pressures.

State	κ	P (kPa)	Φ	z^*	x_0 (%)
6	180900	1	0.593 ± 0.001	4.55 ± 0.02	11.1 ± 0.6
7	1809	10^3	0.596 ± 0.001	5.03 ± 0.04	6.2 ± 0.4
8	389.7	10^4	0.606 ± 0.001	5.48 ± 0.04	3.0 ± 0.3

belled 6 to 8) are along the same compression curve as state 2 of Tab. 1. The radius of the contact surface between the grains in the prestressed initial configurations, b , is related to the normal force F_N by the laws of Hertzian contact elasticity [5]. Given relation $\langle F_N \rangle = \pi d^2 P / (z\Phi)$ [18] of average normal contact force $\langle F_N \rangle$ to the pressure, a characteristic value of b is directly related to κ , the reduced stiffness as defined in (16):

$$\langle b^3 \rangle^{1/3} = \kappa^{-1/2} \left(\frac{3\pi}{z\Phi} \right)^{1/3} R. \quad (18)$$

This gives values of b/R of order 0.02 for $\kappa \simeq 8400$, up to ~ 0.07 for the highest applied pressure (state 8).

3.2. Cement bond network morphology

Intergranular cementation is assumed to take place while the systems of Tabs. 1 and 2 maintain their structure, subjected to the same isotropic pressure. For *bridging cementation* (Fig. 1a) it is considered that all grain pairs initially separated by a distance below h_0 are connected by a bond of equal radius a . As a consequence of distant bonding, the coordination number increases with threshold h_0 , as shown in Fig. 5. This also applies to the grain coating case on replacing h_0 by 2Δ . For h_0 above about $0.05d$, the obtained coordination number z is determined by the solid fraction of the grain pack: state 3, which is denser, has more bonds per grain than (looser) state 4, despite its smaller contact coordination number, and as many as state 5 (which has about the same density). Some grains are then only connected

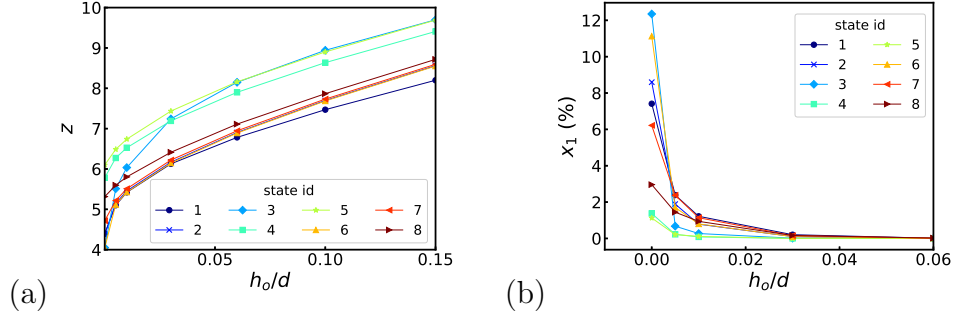


Figure 5: (a) Coordination number z versus bonding range h_0 for bridging cementation (or versus 2Δ for grain coating). (b) Proportion x_1 of rattlers or unstressed grains versus h_0 .

to the rest of the force-carrying structure by a single bond. They may be discarded in the computation of macroscopic elastic properties. Likewise, should any initial rattlers connect and form an “island”, disconnected from the “continent” of force-carrying bonds, the bonds within the island should not contribute to the macroscopic elasticity of the material. The definition of z used here discards connections involving grains that are not on the main force-carrying network. Such grains are counted with the rattlers – they contribute to the rattler fraction x_1 as plotted in Fig. 5b. We keep defining a rattler-corrected coordination number, $z^* = z/(1 - x_1)$, for cemented packs.

Different bond radii a were tested, corresponding to ratios $\alpha = a/R$ taking values ranging from 0.06 to 0.6.

The grain coating model (Fig. 1b) assumes a cement layer of uniform thickness Δ to cover all bead surfaces. Values of Δ/R ranging from 10^{-3} to 0.055 were employed. This corresponds, for contacting grains, to cylindrical bond radii a_0 (Eq. 15) ranging between $0.0447R$ and $0.336R$.

The cement volume may be analytically calculated for both morphologies, whence the cement saturation S (ratio of cement volume to initial pore volume), related to morphological parameters as indicated in Appendix B.

3.3. Bond parameters and bond stiffnesses

In view of the application to gas hydrates, we choose values $G_c = 3.55$ GPa and $\nu_c = 0.3$ [12]. This corresponds to $\Lambda_n \simeq 0.103$ and $\Lambda_t(1 - \nu_g/2) \simeq 3.57 \times 10^{-2}$, and thus to β_n ranging from 0.58 to 5.8 and β_t from 1.68 to 16.8. Ratio k_t/k_n is plotted versus α , with those values of Λ_n and $\Lambda_t(1 - \nu_g/2)$, in Fig. 6. In view of Fig. 3, the normal bond stiffnesses are somewhat lower than

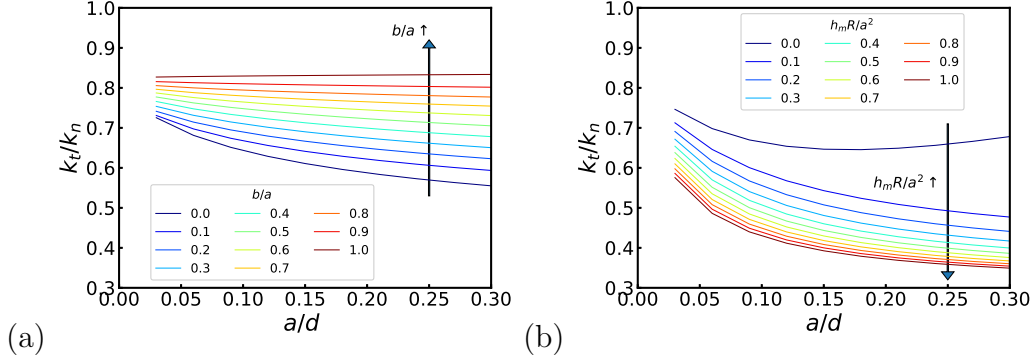


Figure 6: Stiffness ratio k_t/k_n of bonds reinforcing preloaded contacts (a) for different values of b/a , and joining initially distant grains (b), for different gaps h_m , versus a/d .

the rigid punch limit, but of the same order of magnitude (with k_n about 30% lower than k_n^R with the largest bond radii, in the case of initially barely contacting grains, a smaller difference for prestressed contacts, and differences approaching 50% for the largest gaps h_m). The cement bonds should thus be regarded as neither very stiff nor very soft compared to the grains. As to ratio k_t/k_n , it tends to decrease gradually from its rigid limit value $(2 - 2\nu_g)/(2 - \nu_g) \simeq 0.824$ for thicker connections (larger a/d), especially for gap-bridging bonds (larger h_m).

Variations of the mean bond stiffnesses, $\langle k_n \rangle$ and $\langle k_t \rangle$ are shown in Fig. 7. While stiffnesses increase (almost linearly) with the radius a of the cylindrical

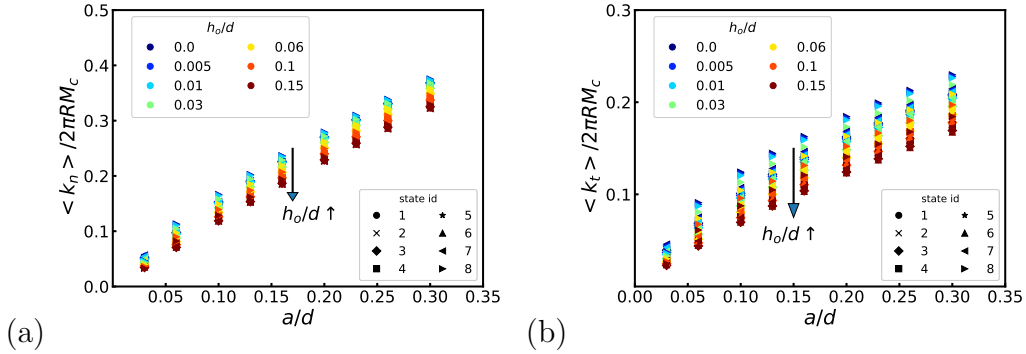


Figure 7: Average bond normal stiffness, $\langle k_n \rangle$ (a) and $\langle k_t \rangle$ (b), normalized (somewhat arbitrarily) by $2\pi R M_c$, versus bond radius a , for different cementation distances h_0 (colors from blue to red and brown).

bonds, an increase in the threshold distance h_0 will result in more numerous

bonds (see Fig. 5), but somewhat softer on average, since connections at larger distance are less stiff (see Fig. 3).

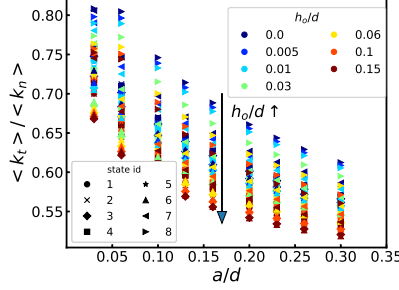


Figure 8: Ratio of average tangential stiffness (Fig. 7b) to average normal stiffness (Fig. 7a) versus a/d .

The tangential stiffness, on average, increases slower than the normal one with bond radius a (as shown by the decreasing trend in Fig. 6). This is apparent in Fig. 7, showing a more conspicuously sublinear trend is the increase of $\langle k_t \rangle$ with a , and is directly shown in Fig. 8, a plot of ratio $\langle k_t \rangle / \langle k_n \rangle$ versus a/d for all available states and different values of h_0 . This ratio becomes smaller for larger bonding range h_0 , and also varies with the initial state.

The macroscopic moduli were also calculated for a stiffer cement, with $G_c = G_g = 26.9$ GPa, such that one has $\Lambda_n \simeq 0.52$ and $\Lambda_t(1 - \nu_g/2) \simeq 0.27$, whence β_n between 0.115 and 1.15 and β_t between 0.222 and 2.22. The resulting stiffnesses are then close to the rigid punch limit (Eqns. 7 and 11).

3.4. Elasticity of cemented packs

We now report measurements of the bulk (K) and shear (G) moduli of cemented packs, depending on cement morphology and elasticity, for each of the 8 initial states. Those moduli are obtained on numerically solving linear systems of equations involving the stiffness matrix of the contact network, in which the degrees of freedom pertaining to pairs of grains joined by a cement bond are coupled, involving the stiffness coefficients k_n , k_t , k_r and k_p as defined in Sec. 2.2. Such calculations are described, e.g., in Refs. [21, 24].

3.4.1. Influence of rolling and pivoting stiffnesses

The influence of the coefficient c_0 of rolling and pivoting stiffness defined in Eq. 13 was tested on comparing values of macroscopic bulk and shear

moduli obtained with $c_0 = 1$, denoted as K_1 and G_1 , to the analogous results K_0 and G_0 obtained with $c_0 = 0$. Fig. 9, in which ratios K_1/K_0 and G_1/G_0 are plotted versus rattler-corrected coordination number z^* (which varies according to initial state and bonding range h_0), makes it quite conspicuous that the stiffening effect of angular elasticity is negligible for well connected bond networks and/or small bond radius a , but very strongly increases the shear modulus of poorly coordinated ones, which is more than doubled for $a/R = 0.2$. Bulk moduli also exhibit a small c_0 dependence for small z^* and large a .

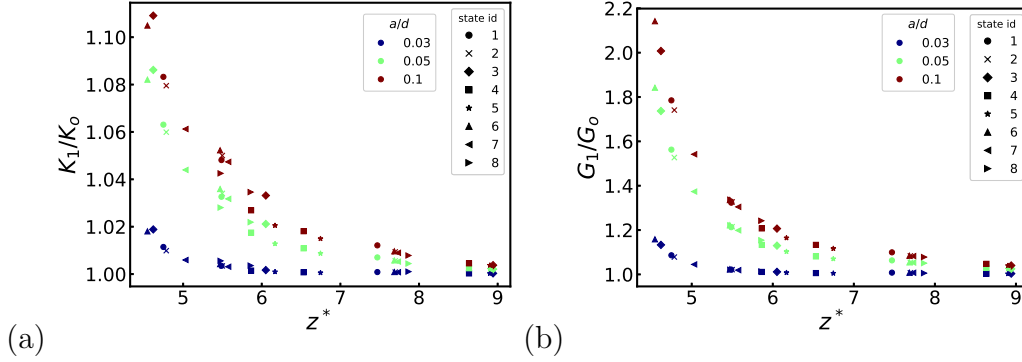


Figure 9: Ratios K_1/K_0 (a) and G_1/G_0 (b), of moduli computed with $c_0 = 1$ to their counterparts evaluated with $c_0 = 0$ versus rattler-corrected coordination number. Color-coded (online) a/d values grow from bottom to top data points.

Interestingly, for the grain coating morphology, the largest values of a also correspond to the largest values of h_0 , thereby increasing the coordination number, and all moduli are quite insensitive to the adopted value of c_0 . In sample series 3, for instance (one of the most sensitive to c_0 for the bridging morphology, because of its small initial coordination number), ratio K_1/K_0 remains below 1.01 and G/G_0 does not exceed 1.06 through the whole range of simulated cement layer thickness Δ .

In the following we adopt rolling and pivoting stiffnesses given by Eq. 13, with value $c_0 = 1^2$, unless specified otherwise, bearing in mind that the shear moduli in some systems with low coordination numbers are sensitive to this choice, in the bridging cementation case

²Some numerical calculations of the pivoting stiffness in the range of parameters β_t , b/a and $h_m R/a^2$ of our study give values of c_0 distributed between 0.8 and 1.

3.4.2. Macroscopic moduli

Effect of morphology and saturation. A comparison of cement saturation-dependent moduli between both cement morphologies, in the case of initial state 4 (Tab. 1), is presented in Fig. 10, for both cases of intergranular

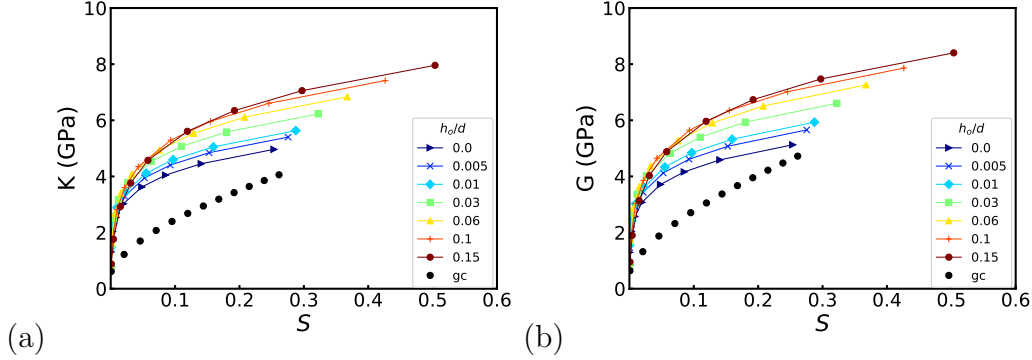


Figure 10: Macroscopic moduli, K (a) and G (b) versus cement saturation of pore space for initial state 4. Bridging cementation results are shown for different cementation distances h_0 , while coating cementation results (gc) form a one-parameter set.

cementation and grain coating (gc). Grain coating results only depend on one parameter (coating thickness Δ) while bridging cementation data depend on both a and h_0 . As expected, moduli strongly increase with diameter a , which causes much of the saturation increase in each of the curves of Fig. 10. The ratios of K and G to their values in the uncemented material under 100 kPa ($\kappa \simeq 8400$) increase from 1 to more than 50 in the investigated range. Fig. 10 makes it clear that cement saturation alone does not determine the material elastic properties, which depend on the cement morphology: moduli differ significantly for the grain coating morphology compared to the grain bridging one, and in this latter case also depend on both parameters h_0 and a (except perhaps for large bonding distances, $h_0 > 0.1d$). For a given saturation, the grain coating morphology yields much smaller moduli, as most of the cement does not contribute to intergranular bonding.

Influence of stress level before cementation. The elastic moduli of unbonded granular materials, as shown in many experiments [20, 28, 29], and numerical simulations [21], are strongly pressure-sensitive, primarily because of the non-linear (Hertz) elasticity of the contact law, and also, to some extent, because of the structural changes – mainly in the coordination number – induced by compression. Our study, with initial states 6, 2, 7 and 8 corresponding to

the same initial material state along its isotropic compression curve (pressure P growing as indicated in Tab. 1 and Tab. 2) provides indications as to a possible influence of the confining stress in the granular material in which cementation takes place. This effect is shown in Fig. 11: it is quite notable

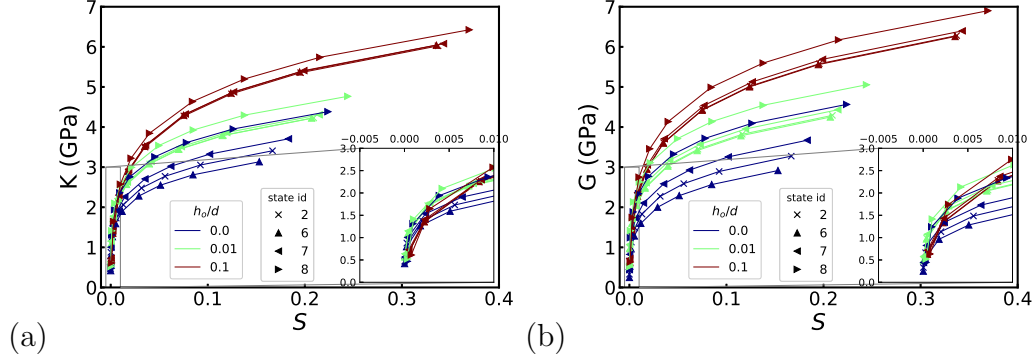


Figure 11: Macroscopic moduli, K (a) and G (b) versus cement saturation of pore space for initial states 6, 2, 7, and 8. Three (color-coded online) sets of values for different h_0 .

for the highest pressure investigated (10 MPa) whatever the other parameters, and also for small cement saturation and/or bonding range at lower pressure.

Variations with average stiffness and bond density. Fig. 12 gathers the results for all 8 initial states in which dimensionless ratios $Kd/\langle k_n \rangle$ and $Gd/\langle k_n \rangle$ are plotted versus product $z\Phi$. This product is proportional to the bond density, and varies mostly because of changes in z , between different initial states (Tabs. 1 and 2), and as a consequence of cementation (Fig. 5). Such a representation appears to capture the essential parameter dependence of the macroscopic elastic results: all the data, for both morphologies, gather near a single curve, with a nearly linear trend.

Shear moduli: singular limit and influence of angular elasticity. The scatter is more pronounced for shear moduli (with systematically larger values for larger bond radius a at small $z\Phi$). To some extent, this may be attributed to a greater influence of tangential stiffnesses in the shear response. As shown in Fig. 8, tangential stiffnesses k_t are not proportional to k_n . But this is also due to the greater influence of angular stiffnesses, which significantly increases the shear moduli of systems with small coordination and large bond radius a (Sec. 3.4.1). The trend for the variations of G at low $z\Phi$ is similar to

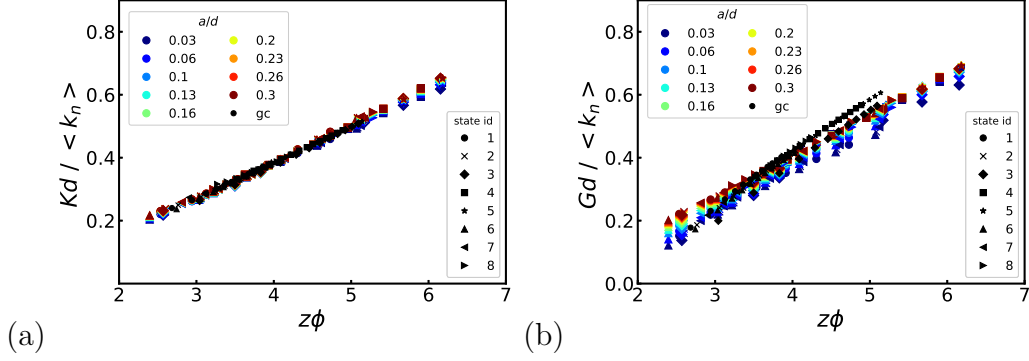


Figure 12: Macroscopic moduli, K (a) and G (b), normalized by $\langle k_n \rangle / d$, versus $z\Phi$ for the complete parameter range and both morphologies.

the shear modulus singularity of non-bonded granular materials [21, 46, 24]. In the absence of rolling and pivoting stiffnesses, G tends to vanish (as argued theoretically [47]), together with the degree of force indeterminacy, as the rattler-corrected coordination number z^* approaches 4 (with a very small correction due to 2-coordinated grains, see [21]). The greater sensitivity of shear modulus G to angular elasticity, as reported in Sec. 3.4.1, is related to this singularity: a finite shear modulus would be restored by rolling and pivoting contact stiffnesses even in the limit of $z^* = 4$, which ceases to correspond to “barely rigid” structures (devoid of force indeterminacy) if angular elasticity is present. Thus, ratio G_1/G_0 , as shown in Fig. 9, is expected to diverge in the limit of $z^* \rightarrow 4$.

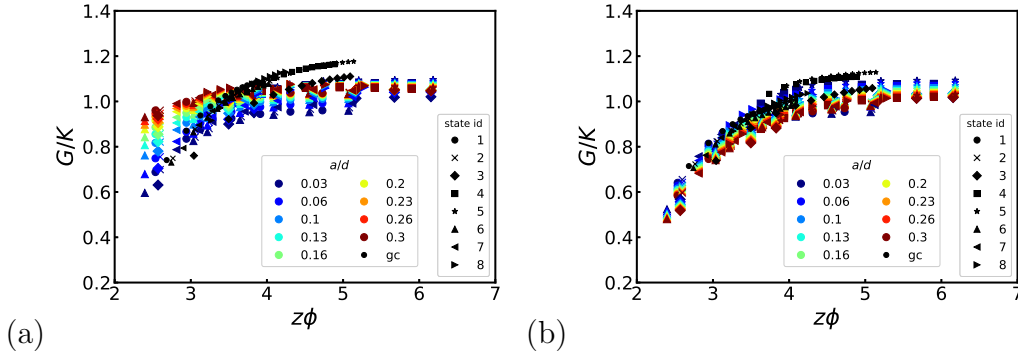


Figure 13: Ratio G/K for moduli computed with $c_0 = 1$ (a) and $c_0 = 0$ (b) versus $z\Phi$, for the complete data set.

Shear to bulk moduli ratios, Poisson ratio. As a consequence of the somewhat different dependence of G and K on the initial structure and the model parameters, their ratio G/K varies, as shown in Fig. 13, with a growing trend with the bond density. Its extreme values, from about 0.6 for some poorly coordinated systems up to 1.1 or 1.15 for the highest connectivities, respectively correspond to values of macroscopic Poisson ratios $\nu^* = (K - 2G/3)/(2K + 2G/3)$ of $\simeq 0.25$ and $\simeq 0.08$ – 0.1 – a behavior reminiscent of unbonded materials [21]. For small contact densities, the range of observed values of G/K reflects the greater influence on G of angular stiffnesses (choice of c_0 , see Fig. 9): the shear moduli corresponding to the largest values of a/R are significantly larger. As shown in Fig. 13b, this disparity tends to vanish on setting c_0 to zero.

Moduli for very stiff bonds. The difference between our results and the stiff bond (rigid punch) limit may be appreciated in Fig. 14, showing the ratios of moduli K and G to their values with the same structures, but with the cement chosen as stiff as the grain material. As expected given the values of

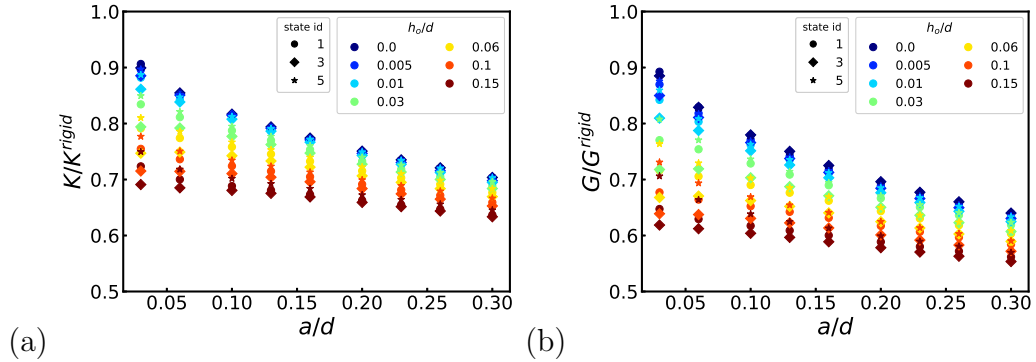


Figure 14: Ratios K/K^{rigid} (a) and G/G^{rigid} (b), of the measured moduli to their values in the stiff cement case, versus a/d for different (color-coded online, increasing from top to bottom data points) bonding ranges h_0 , in sample series 1, 3, and 5.

k_n/k_n^R and k_t/k_t^R discussed in Sec. 3.3, the computed macroscopic moduli, using the bond elasticity model of Secs. 2 and Appendix A are notably lower, with a relative difference ranging between 10% and 40%, than the ones predicted using the simple laws (7) and (11) applicable in the rigid bond limit. Those nevertheless provide a rough estimate (correct to within 20 or 25% if $a/R \leq 0.2$ and $h_0/d \leq 0.03$).

4. Approximate prediction schemes

We now test the simplest approaches designed to predict the values of the effective macroscopic moduli of cemented granular materials. Leaving aside the more elaborate scheme proposed by La Ragione and Jenkins [31], we test the simple Voigt or “effective medium” approximations [48, 30, 21, 22], which provide upper bounds [21] to moduli K and G ; we exploit the available Reuss-type lower bound to K [21]; and we finally discuss the approaches proposed by Dvorkin *et al.* [14, 2], which attempt a prediction of the moduli from the sole knowledge of the cement saturation.

4.1. Voigt mean field predictions

The Voigt predictions for the bulk modulus K and the shear modulus G of an isotropic assembly of equal-sized spherical beads of radius R are respectively denoted below as K^V and G^V . Using the values of an affine displacement field within a homogeneous elastic continuum as trial displacements of grain centers, they depend on coordination number z , grain solid fraction Φ , on some averages over all intergranular contacts or bonds involving center-to-center distance r , and on normal (k_n) and tangential (k_t) bond stiffnesses, as follows:

$$\begin{aligned} K^V &= \frac{z\Phi}{24\pi R^3} \langle k_n r^2 \rangle \simeq \frac{z\Phi}{6\pi R} \langle k_n \rangle \\ G^V &= \frac{z\Phi}{8\pi R^3} \left[\frac{1}{5} \langle k_n r^2 \rangle + \frac{3}{10} \langle k_t r^2 \rangle \right] \simeq \frac{z\Phi}{2\pi R} \left[\frac{1}{5} \langle k_n \rangle + \frac{3}{10} \langle k_t \rangle \right]. \end{aligned} \quad (19)$$

In Eqs 19, the rightmost expression exploits the approximation $\langle k_{n,t} r^2 \rangle \simeq 4R^2 \langle k_{n,t} \rangle$, for which the maximum relative error is about 4% in our numerical results. Fig. 15 shows ratios K/K^V and G/G^V plotted versus rattler-corrected coordination number z^* . Note that the influences of the average bond stiffness and of the density of contacts $z\Phi$ (as evidenced in the results of Fig. 12) are eliminated on computing such ratios, since the Voigt estimates are proportional to both quantities. Fig. 15 shows those ratios to be largely determined by z^* , the farther from 1 the smaller z^* , as observed in numerical investigations of non-bonded granular materials [46, 21, 22, 24] and in some models for sintered packs [49]. The Voigt estimate K^V provides a fair approximation of the bulk modulus K , although overestimating it by about 25% for small z^* . However, G^V may overestimate the shear modulus G by a factor as large as 3, for poorly coordinated systems [21]. Fluctuations, i.e.,

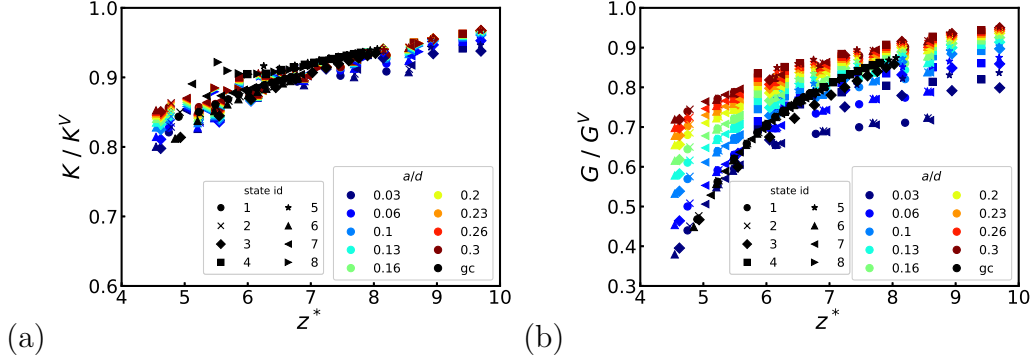


Figure 15: Ratios K/K^V (a) and G/G^V (b), of the measured moduli to their Voigt estimates, versus rattler-corrected coordination number z^* , for the whole data set. Values increase for larger (color-coded online) bond radius.

errors made on assuming affine displacements fields or homogeneous strains as in the Voigt approximation, are known [21] to increase in networks with small force indeterminacy (low z^*). The Voigt estimate does not capture the marked decrease of shear modulus G for small z^* in granular materials joined by thin bonds (small a/R), and it is of course unable to predict its sensitivity to angular elasticity for larger bond radii – expression (19) of the shear modulus, based on trial displacements and involving no grain rotation [21], ignores rolling and pivoting stiffnesses. Fig. 16 shows that the error

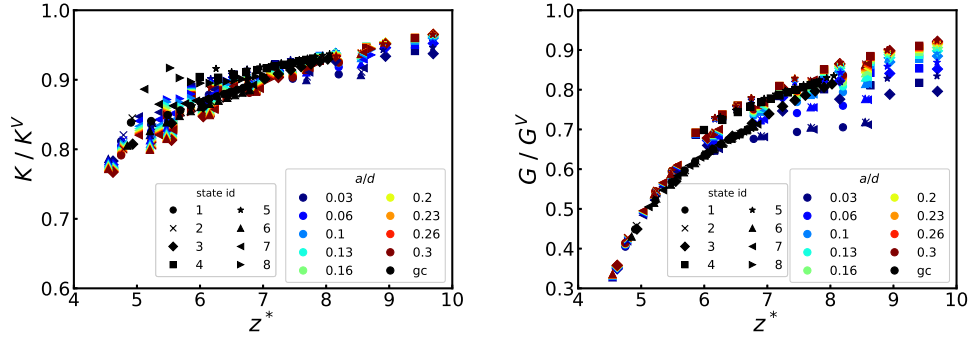


Figure 16: Same as in Fig. 15, for moduli computed without angular elasticity ($c_0 = 0$).

of Voigt estimates gets smaller in cases for which angular stiffnesses increase the shear modulus. It also shows that, on suppressing angular elasticity (setting $c_0 = 0$), the values of moduli corresponding to the same coordination

number and average normal stiffness are close together, whence common values of K/K^V and G/G^V . On setting $c_0 = 0$, all shear moduli of poorly coordinated systems, whatever the bond radius a , are much lower than the Voigt prediction (and tend to vanish as z^* approaches 4).

4.2. A lower bound for the bulk modulus

As explained in Refs. [21, 50], a lower bound may be obtained for the bulk modulus of isotropically prestressed granular assemblies, on exploiting the knowledge of the distribution of contact forces (which is well documented [18] and has been rather thoroughly investigated, see *e.g.* Ref. [51]). Let us denote as $\langle k_n \rangle_c$ the average of normal cement bond stiffnesses formed in *contacting* grain pairs, and write z_c for the coordination number in the granular pack prior to cementation. The lower bound K^R to the bulk modulus involves a certain dimensionless quantity \tilde{Z} , of order 1, characterizing the initial distribution of contact forces (F_N, F_T) in the prestressed, uncemented system under isotropic pressure. \tilde{Z} also involves the stiffnesses of the cemented material, and is defined as

$$\tilde{Z} = \frac{\langle k_n \rangle_c}{\langle F_N \rangle^2} \left\langle \frac{F_N^2}{k_n} \left[1 + \frac{k_t}{k_n} \frac{F_T^2}{F_N^2} \right] \right\rangle. \quad (20)$$

Then, K^R simply writes

$$K^R = \frac{z_c \Phi}{6\pi \tilde{Z} R} \langle k_n \rangle_c. \quad (21)$$

One recognizes formula (19) for the Voigt estimate K^V , in which product $z \langle k_n \rangle$ is replaced by $z_c \langle k_n \rangle_c$, and the result is divided by \tilde{Z} . Assuming no correlation between stiffness k_n (after cementation) and contact forces before cementation, and noting that k_t/k_n is smaller than 1 (see Figs. 6 and 8), while $(F_T/F_N)^2$ is bounded by the square of the friction coefficient, $\mu^2 = 0.09$, \tilde{Z} is close to ratio $\langle F_N^2 \rangle / \langle F_N \rangle^2$, which is typically of order 1.5 [18, 52]. Thus the relative difference between lower bound K^R and upper bound K^V is of order 30-40% for vanishing bonding range ($z = z_c$). This shows that the error in the Voigt or Reuss estimates of K cannot be very large.

The ratio K^R/K of the Reuss-estimated to the measured values of bulk moduli is plotted in Fig. 17 versus z^* . The Reuss estimate is based on the force distribution within the initial contact network. It does not change as z increases with cement saturation, and is well below the exact moduli for large S and large z . It is close to the measured value in the range of small z^* , for which the error in the Voigt estimate is the largest.

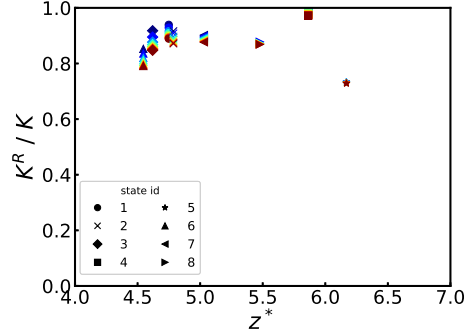


Figure 17: Ratio K^R/K versus z^* , in small coordination systems.

4.3. Mean-field approaches for the elastic moduli of hydrate-bearing sands

The prediction of elastic moduli of model hydrate-bearing granular soils by Dvorkin and Nur [2] (DN in the following), and Dvorkin *et al.* [15] is a version of the Voigt estimation scheme for spherical bead packs formulated for situations in which detailed microstructural information – coordination number, distribution of distances between neighboring grains that might be bridged by cement bonds – are not available. The scheme is explicitly presented in Ref. [2], expressed in readily exploitable analytical form³. The value of $\alpha = a/R$ is deduced from cement saturation S on inverting the relations written in Appendix B, to leading order in α . Then an estimate of mean normal, k_n^D , and tangential, k_t^D , stiffnesses, is obtained by the fitting formulae given in Ref. [2], which are adequate for cemented *contacts* (i.e., $h_0 = 0$, $b = 0$), depending on this particular value of α , assumed representative of all bonds. As to coordination numbers, the authors of Ref. [2], based on the knowledge one had in the time of its publication, use values between 8 and 9, which are the largest ones observed in our numerical study, for the largest bonding range h_0 or coating layer thickness Δ . This is contradictory with the use of bond stiffness values for $h_0 = 0$.

In the case of bridging cementation, numerical simulations provide a two-parameter set of results, depending on a and h_0 , and moduli might differ for the same value of S , according to the combination of a and h_0 (Fig. 10), a feature which the DN scheme necessarily ignores, as well as it overlooks the

³It was pointed out, though, in Ref. [1], that those results could lack accuracy for quite different cement and grain moduli, i.e. for Λ_n , Λ_t significantly smaller or larger than 1.

possible increase of z with S . The assumed relationship between a and S is also inadequate in general: Appendix B shows that the contribution of a growing z to the cement volume is important.

It is more convenient to assess the performances of the DN scheme for the grain coating morphology, as both numerical results and DN predictions are then single-valued functions of Δ or of S . The comparison of DN-predicted to measured moduli is carried out in both cases of an initially well coordinated grain pack (state 4) and a poorly coordinated one (state 3). In both cases, the

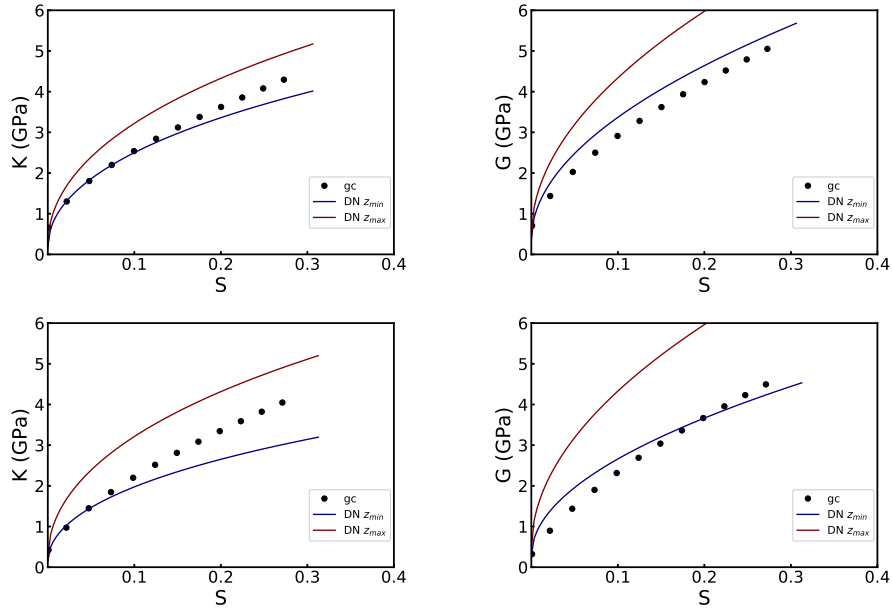


Figure 18: Bulk (left) and shear (right) moduli versus cement saturation for gain coating morphology, with initial states 4 (top plots) and 3 (bottom ones), compared to Dvorkin-Nur (DN) predictions [2]. Data points: numerical results. Upper line: DN prediction for maximum (large Δ) coordination number. Lower line: DN prediction for initial (minimum) coordination number.

DN formula is tested (Fig. 18) either with the initial (contact) coordination number (z_{\min} , 4.83 for state 3 and 5.86 for state 4) or with the largest value of z (z_{\max} , close to 8 in both states 3 and 4) reached for the largest saturation (largest Δ). Note first that, only knowing the solid fraction of states 3 and 4, it is not obvious to predict that z_{\min} is lower in the denser one (which is state 3, see Table 1). The usual DN scheme is applied on postulating a value close to z_{\max} . Fig. 18 shows that the model predicts correct orders

of magnitude, but that the choice of an appropriate coordination number is rather problematic: too small bulk moduli are obtained with z_{\min} for $S \geq 0.15$, but the choice of the cemented high value z_{\max} overestimates K in all cases. As to the shear modulus, it is considerably overestimated, even using the value z_{\min} in the DN scheme, except for state 3 at high enough saturation, $S \geq 0.2$.

The constant value $z = 8$ or $z = 9$ generally adopted by DN is thus quite inadequate, but no other constant value would correctly describe the data throughout the saturation range, because the increase of z with S is an important physical feature of cemented grain packs. As another possible source of problems, one may also note that the predicted values of $\langle k_n \rangle$ and $\langle k_t \rangle$ based on the assumptions and parametrization of the DN scheme sometimes differ (by as much as 50%) from our own results, partly due to the contribution of precompressed contacts and of gap-bridging bonds.

5. Conclusions

We now sum up the main results of our study, then briefly evoke some possible perspectives.

We first generalize the model of Ref. [1] to deal with cylindrical cement bonds of radius a , joining initially non-contacting spherical grains of radius R , separated by a small distance h_m , and to precompressed contacts, where grains touch by a disk of radius b . The numerical solutions for normal, k_n , and tangential, k_t , bond stiffnesses, are written in parametrized forms (Appendix A), suitable for implementation in numerical simulations of large grain packs, as functions of the two relevant dimensionless parameters: β_n or β_t (Eqns. 6 and 10) combining material elasticity with geometric parameter $\alpha = a/R$, as remarked in Ref. [1], on the one hand; and, on the other hand, either on b/a , the ratio of the initial contact region radius in the prestressed state of the grain pair to the bond radius, in the precompressed case, or on the dimensionless distance $h_m R/a^2$ between grain surfaces prior to cementation, for distant bonding. With elastic properties of grains and cement corresponding to hydrates forming in sands [12], the stiffness of cement bonds is notably below the rigid cement limit, albeit of the same order of magnitude (Fig. 14).

In a second stage this local bond model is used in DEM-simulated packs of spherical balls, equilibrated under isotropic pressure, and forming different structures, dense or loose, with high or low coordination numbers. Two

different cement morphologies are studied, resulting in different relations between bond geometries and average cement saturation: bonding cementation, and grain coating. The macroscopic moduli are essentially determined by the average bond stiffnesses, depending on the distribution of geometric parameters and on the material stiffnesses; and by the cement bond network coordination number, which determines the bond density. Just like for ordinary, non-cemented granular materials [21, 24], bonded grain packs with low coordination tend to exhibit rather singular, anomalously small shear moduli. Those are strongly overestimated by a Voigt estimate, while the Voigt approximation for bulk moduli is typically within 25% of the measured value. Thanks to the available Reuss estimate, which exploits the knowledge of the force distributions in compressed non-cemented grain packs, the error of the Voigt estimate of the bulk modulus is controlled for small saturation. Our study reveals that the angular stiffnesses, expressing elastic resistance to rolling or pivoting of bonded pairs of grains, do influence the macroscopic moduli (especially the shear moduli) provided the coordination number is small enough (say, below 5.5) and the bond radius a is large enough (of order $R/10$). This effect disappears for well coordinated bond networks or small bond radii, and does not affect the cemented systems with the grain coating morphology, for which large a values also imply large coordination numbers. Excluding the poorly coordinated systems with large bond radii that are sensitive to angular stiffnesses, it is a fair approximation to regard ratios $dK/\langle k_n \rangle$ and $dG/\langle k_n \rangle$ as single-valued functions of $z\Phi$.

If the coordination number did not change during cementation, keeping its initial, contact value z_c , one might roughly estimate the ratio of the moduli of the cemented material to the initial moduli of the uncemented prestressed granular pack on using the rigid punch limit values of bond stiffnesses k_n and k_t . Those stiffnesses are those of necks of radius a , while the radius b due to grain contact forces similarly determine the uncemented stiffnesses. Thus the ratio of the moduli in the cemented material to their values in the prestressed material prior to cementation would be simply a/b , and typical values of b may be obtained from relation (18). However, one essential contribution of this study, which deals with “realistic” granular microstructures, i.e., assembled by equilibrating granular samples under pressure, is to highlight the crucial role of coordination number z , which changes in the cementation process. Not only does its value before cementation, z_c (varying roughly between 4 and 6 under moderate pressures [18, 43]) strongly influence the moduli at low cement saturation, but the marked increase of z

for growing cement content is a major factor in the increase of macroscopic moduli with cement saturation.

The Dvorkin-Nur scheme used in some geophysical applications is based on the Voigt approximation, and on a representation of the bond network as made of a fixed density of bonds (fixed coordination number) with a radius growing with the cement saturation. While predicting correct orders of magnitude, it suffers from the same shortcomings as the Voigt approximation, and, furthermore, cannot account for the influence of the initial value of the coordination number, and of its increase under growing cement saturation.

In order to obtain better prediction schemes, one should supply more information on coordination numbers, before and after cementation. As pointed out in several numerical studies of granular material elasticity [21, 53, 24], macroscopic moduli indirectly provide useful information on coordination numbers. Thus, available measurements of the granular material elasticity before cementation could be useful. As to the propensity to form additional bonds, thereby increasing the coordination number, for growing cement saturation, X-ray microtomography [54], although it often lacks the accuracy to identify correctly the contact coordination number, could access the desired distribution of intergranular distances (function $z(h)$ of Fig. 5). The network of intergranular bonds in a weakly cemented granular material for which the cement is preferentially deposited on the grain surfaces should be much easier to visualize than the contact network with no cement. In fact, small capillary bonds bridging the grains in the presence of a wetting liquid are successfully observed [55, 56, 57]. Similar observations for solid bonds could contribute to the design of more accurate morphological models, and numerical simulations could be used in conjunction with such experiments in order to test micromechanical approaches. The great advantage of elastic measurements is that they provide a non-destructive mechanical characterization of the material. Further numerical studies could correlate elastic and failure properties, in the hope that measurements of the former could be used to predict the latter. It may also become necessary, rather than postulating some simple but rather arbitrary morphological model for the cement, to design a physical model of cement formation in the pore space.

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Appendix A. Fit function for the elastic response of a bonded contact.

Distant grains

The rigid punch result, (7) or (11), gets corrected by a function g_1^n or g_1^t of the dimensionless parameters:

$$\begin{aligned} k_n &= \frac{k_n^R}{g_1^n}, \\ k_t &= \frac{k_t^R}{g_1^t}. \end{aligned} \tag{A.1}$$

Defining $\beta_n = \alpha/\Lambda_n$, $\beta_t = \frac{\alpha}{\Lambda_t(1-\nu_g/2)}$ and $\gamma = h_m R/a^2$, one finds good approximations of the following form:

$$g_1(\beta, \gamma) = c_1(\gamma) + d_1(\gamma)\beta + e_1(\gamma)\beta^2 + h_1(\gamma)\beta^3, \tag{A.2}$$

with coefficients of the powers of β_n or β_t given as functions of γ alone. For the normal stiffness, we found:

$$\begin{aligned} c_1^n &= 0.9769 + 0.0607 \gamma^{0.0959} \\ d_1^n &= 0.1147 + 0.4051 \gamma \\ e_1^n &= -0.0054 + 0.0036 \gamma^{0.0869} \\ h_1^n &= 0.00009 - 0.00006 \gamma^{0.1060}. \end{aligned} \tag{A.3}$$

As to the tangential case, function $g_1^t(\beta, \gamma)$ is obtained in the form (A.2) with

$$\begin{aligned} c_1^t &= 0.9705 + 0.7682 \gamma^{0.0910} \\ d_1^t &= 0.1356 + 0.4765 \gamma \\ e_1^t &= -0.0073 + 0.0052 \gamma^{0.0731} \\ h_1^t &= 0.00013 - 0.00009 \gamma^{0.0794}. \end{aligned} \tag{A.4}$$

As long as the dimensionless parameters satisfy $\beta_{n,t} \leq 30$ and $\gamma = \frac{h_m R}{a^2} \leq 750$, the relative error made on replacing the calculated stiffness k_n or k_t by the value predicted via Eqs. (A.1), and (A.2), with (A.3) or (A.4), remains below 5%.

Precompressed grains

A correction function g_2 is similarly defined, such that stiffnesses are given by $k_n = k_n^R/g_2^n$ and $k_t = k_t^R/g_2^t$, depending again on $\beta_n = \alpha/\Lambda_n$ or $\beta_t = \frac{\alpha}{\Lambda_t(1-\nu_g/2)}$ but now on $\delta = b/a$. g_2 may be expressed as

$$g_2(\beta, \delta) = c_2(\delta) + d_2(\delta)\beta^{e_2}, \quad (\text{A.5})$$

with, for the normal case,

$$\begin{aligned} c_2^n &= 1.0037 - 0.4542\delta + 0.7588\delta^2 - 0.3034\delta^3 \\ d_2^n &= 0.1638 + 0.7295\delta - 1.7095\delta^2 + 0.8134\delta^3 \\ e_2^n &= 0.7216 - 1.2590\delta + 1.1270\delta^2 - 0.2670\delta^3, \end{aligned} \quad (\text{A.6})$$

and in the tangential case:

$$\begin{aligned} c_2^t &= 0.9974 - 0.1447\delta + 0.1980\delta^2 - 0.0461\delta^3 \\ d_2^t &= 0.2086 + 0.2350\delta - 0.8396\delta^2 + 0.3906\delta^3 \\ e_2^t &= 0.7048 - 0.6034\delta + 0.0604\delta^2 + 0.2614\delta^3. \end{aligned} \quad (\text{A.7})$$

Within the parameter range $\beta_{n,t} \leq 35$ and $\delta = \frac{b}{a} \leq 1$, the relative error made on replacing the calculated stiffness by the value obtained on fitting g_2 by its form (A.5) using (A.6) or (A.7) remains below 4% in the normal case and below 3% in the tangential case.

Appendix B. Cement saturation

We express here the cement saturation, S , as a function of solid fraction Φ , coordination number z , and parameters a and h_0 in the case of bridging cementation, or Δ in the case of grain coating. The calculation of S involves function $z(h)$, as plotted in Fig. 5a, and its derivative $z'(h)$. We denote as z_c the contact coordination number (the value of z before cementation), while notation z is kept for the coordination number after cementation: $z(h_0)$ in the bridging cementation case, or $z(2\Delta)$, for grain coating. We also write z_d for the difference $z - z_c$.

For bridging cementation, the cement volume is obtained as the sum of the bridge volumes. Distinguishing the contributions of cemented contacts, S_{con} and bridges between initially distant grains, S_{dis} , we neglect the effect of the grain deformation in the contact region to compute the former as

$$S_{\text{con}} = \frac{3\Phi z_c}{8(1-\Phi)} \left[\frac{a^4}{2R^4} + \frac{a^6}{12R^6} \right], \quad (\text{B.1})$$

while the latter reads:

$$S_{\text{dis}} = \frac{3\Phi a^2}{8(1-\Phi)R^3} \left\{ z_d \left[\frac{a^2}{2R} + \frac{a^4}{12R^3} \right] + \int_0^{h_0} h z'(h) dh \right\}. \quad (\text{B.2})$$

The integral may be evaluated on exploiting the power law variation of $z(h)$ for small h [18]: one has

$$z(h) = z_c + Ah^m, \quad (\text{B.3})$$

with an exponent m in range 0.4-0.6, depending on the initial state. $S = S_{\text{con}} + S_{\text{dis}}$ rearranges into:

$$S = \frac{3\Phi}{8(1-\Phi)} \left\{ z \left(\frac{a^4}{2R^4} + \frac{a^6}{12R^6} \right) + z_d \frac{m}{m+1} \frac{a^2 h_0}{R^3} \right\} \quad (\text{B.4})$$

The first term of (B.4) is analogous to S_{con} , coinciding with expression (B.1) in which the cemented coordination number, z , replaces the contact coordination number, z_c . As h_0 grows from zero, this term expresses the saturation increase due to the formation of additional bonds. The second term represents the effect of the larger volume of these additional bonds joining initially distant grains, and its ratio to the first one is

$$\frac{2m}{m+1} \frac{z_d}{z} \frac{h_0 R}{a^2},$$

which is often (but not always) small with our parameter choice.

The Dvorkin-Nur scheme, using coordination number z_c , relates S to α , on setting S_{dis} to zero (imposing $h_0 = 0$, a strong morphological assumption) and keeping only the first term, of order $(a/R)^4$, in S_{con} , whence a small error. On using z instead of z_c , it would more correctly relate S to α (since S_{con} is replaced by the first term of (B.4), but loses then accuracy in the estimation of $\langle k_n \rangle$, because of assumption $h_0 = 0$).

In the grain coating case, the saturation is computed on adding up the volume of the cement layer covering each grain, corrected for the intersection of the layers in the contact or narrow intergranular gap regions. Thus

$$S = S_0 - S_1, \quad (\text{B.5})$$

with

$$S_0 = \frac{\Phi}{1-\Phi} \left[3 \frac{\Delta}{R} + 3 \frac{\Delta^2}{R^2} + \frac{\Delta^3}{R^3} \right]. \quad (\text{B.6})$$

S_1 involves the contribution of contacts and another one of noncontacting pairs with intersecting cement layers, and may be evaluated on exploiting the power law form (B.3) of $z(h)$. But the values of Δ used in the present study are such that the relative errors on $S(\Delta)$, on using approximation $S \simeq S_0$, never exceeds 3%. This approximation is used in the DN scheme, which then obtains the bond radius a from relation (15).

References

- [1] V. Langlois, Elastic behavior of weakly cemented contact, *International Journal for Numerical and Analytical Methods in Geomechanics* 39 (8) (2015) 854–860.
- [2] J. Dvorkin, A. Nur, Elasticity of high-porosity sandstones: Theory for two North Sea data sets, *Geophysics* 61 (5) (1996) 1363–1370.
- [3] B. Andreotti, Y. Forterre, O. Pouliquen, *Granular Media: Between Fluid and Solid*, Cambridge University Press, Cambridge, UK, 2013.
- [4] F. Radjaï, J.-N. Roux, A. Daouadji, Modeling Granular Materials: Century-long Research across Scales, *ASCE Journal of Engineering Mechanics* 143 (4) (2017) UNSP 04017002.
- [5] K. L. Johnson, *Contact Mechanics*, Cambridge University Press, 1985.
- [6] D. Bernache-Assolant (Ed.), *Chimie-physique du frittage*, Hermès, 1993.
- [7] X. Liu, C. L. Martin, G. Delette, D. Bouvard, Elasticity and strength of partially sintered ceramics, *Journal of the Mechanics and Physics of Solids* 58 (6) (2010) 829–842.
- [8] C. L. Martin, Unloading of powder compacts and their resulting tensile strength, *Acta Materialia* 51 (15) (2003) 4589–4602.
- [9] C. L. Martin, L. C. R. Schneider, L. Olmos, D. Bouvard, Discrete element modeling of metallic powder sintering, *Scripta Materialia* 55 (5) (2006) 425–428.
- [10] J. T. Dejong, K. Soga, E. Kavazanjian, S. E. Burns, L. A. Van Paassen, A. Al Qabany, A. Aydilek, S. S. Bang, M. Burbank, L. F. Caslake, C. Y. Chen, X. Cheng, J. Chu, S. Ciurli, A. Esnault-Filet, S. Fauriel,

- N. Hamdan, T. Hata, Y. Inagaki, S. Jefferis, M. Kuo, L. Laloui, J. Larrahondo, D. A. C. Manning, B. Martinez, B. M. Montoya, D. C. Nelson, A. Palomino, P. Renforth, J. C. Santamarina, E. A. Seagren, B. Tanyu, M. Tsesarsky, T. Weaver, Biogeochemical processes and geotechnical applications: progress, opportunities and challenges, *Géotechnique* 63 (4) (2013) 287–301.
- [11] A. Al Qabany, K. Soga, Effect of chemical treatment used in MICP on engineering properties of cemented soils, *Géotechnique* 63 (4) (2013) 287–301.
- [12] W. F. Waite, J. C. Santamarina, D. D. Cortes, B. Dugan, D. N. Espinoza, J. Germaine, J. Jang, J. W. Jung, T. J. Kneafsey, H. Shin, K. Soga, W. J. Winters, T.-S. Yun, Physical properties of hydrate-bearing sediments, *Reviews of Geophysics* 47 (2009) RG4003.
- [13] J. Dvorkin, G. Mavko, A. Nur, The effect of cementation on the elastic properties of granular material., *Mechanics of Materials* 12 (3-4) (1991) 207–217.
- [14] J. Dvorkin, A. Nur, H. Yin, Effective properties of cemented granular materials, *Mechanics of Materials* 18 (4) (1994) 351–366.
- [15] J. Dvorkin, J. G. Berryman, A. Nur, Elastic moduli of cemented sphere packs, *Mechanics of Materials* 31 (7) (1999) 461–469.
- [16] M. B. Helgerud, J. Dvorkin, A. Nur, A. Sakai, T. Collett, Elastic-wave velocity in marine sediments with gas hydrates: Effective medium modeling, *Geophysical Research Letters* 26 (13) (1999) 2021–2024.
- [17] F. Radjaï, F. Dubois (Eds.), *Discrete-element modeling of granular materials*, Wiley, 2011.
- [18] I. Agnolin, J.-N. Roux, Internal states of model isotropic granular packings. I. Assembling process, geometry, and contact networks, *Phys. Rev. E* 76 (6) (2007) 061302.
- [19] S. Shibuya, F. Tatsuoka, S. Teachavorasinskun, X.-J. Kong, F. Abe, Y.-S. Kim, C.-S. Park, Elastic deformation properties of geomaterials, *Soils and Foundations* 32 (1992) 26–46.

- [20] P.-Y. Hicher, Elastic properties of soils, *ASCE Journal of Geotechnical Engineering* 122 (1996) 641–648.
- [21] I. Agnolin, J.-N. Roux, Internal states of model isotropic granular packings. III. Elastic properties, *Phys. Rev. E* 76 (6) (2007) 061304.
- [22] V. Magnanimo, L. La Ragione, J. T. Jenkins, P. Wang, H. A. Makse, Characterizing the shear and bulk moduli of an idealized granular material, *Europhys. Lett.* 81 (2008) 34006.
- [23] L. La Ragione, V. Magnanimo, Evolution of the effective moduli of an anisotropic, dense granular material, *Granular Matter* 14 (2012) 749–757.
- [24] M. H. Khalili, J.-N. Roux, J.-M. Pereira, S. Brisard, M. Bornert, Numerical simulation of one-dimensional compression of granular materials: II. Elastic moduli, stresses and microstructure., *Physical Review E* 95 (2017) 032908.
- [25] S. N. Domenico, Elastic properties of unconsolidated porous sand reservoirs, *Geophysics* 42 (7) (1977) 1339–1368.
- [26] Y.-C. Chen, I. Ishibashi, J. T. Jenkins, Dynamic shear modulus and fabric: part I, depositional and induced anisotropy, *Géotechnique* 38 (1) (1988) 23–32.
- [27] Y.-C. Chen, I. Ishibashi, J. T. Jenkins, Dynamic shear modulus and fabric: part II, stress reversal, *Géotechnique* 38 (1) (1988) 33–37.
- [28] R. Kuwano, R. J. Jardine, On the applicability of cross-anisotropic elasticity to granular materials at very small strains, *Géotechnique* 52 (2002) 727–749.
- [29] Y. Khidas, X. Jia, Anisotropic nonlinear elasticity in a spherical bead pack: influence of the fabric anisotropy, *Phys. Rev. E* 81 (2010) 021303.
- [30] K. Walton, The effective elastic moduli of a random packing of spheres, *Journal of Mechanics and Physics of Solids* 35 (1987) 213–226.
- [31] L. La Ragione, J. T. Jenkins, The initial response of an idealized granular material, *Proceedings of the Royal Society A* 63 (2079) (2007) 735–758.

- [32] D. O. Potyondy, P. A. Cundall, A bonded-particle model for rock, *International Journal of Rock Mechanics and Mining Sciences* 41 (8) (2004) 1329–1364.
- [33] Y. Wang, F. Alonso-Marroquin, A finite deformation method for discrete modeling: particle rotation and parameter calibration, *Granular Matter* 11 (5) (2009) 331–343.
- [34] L. Brendel, J. Török, R. Kirsch, U. Bröckel, A contact model for the yielding of caked granular materials, *Granular Matter* 13 (6) (2011) 777–786.
- [35] M. Obermayr, K. Dressler, C. Vrettos, P. Eberhard, A bonded-particle model for cemented sand, *Computers and Geotechnics* 49 (2013) 299–313.
- [36] N. J. Brown, J. Chen, J. Y. Ooi, A bond model for DEM simulation of cementitious materials and deformable structures, *Granular Matter* 16 (3) (2014) 299–311.
- [37] Z. Shen, M. Jiang, C. Thornton, DEM simulation of bonded granular material. Part i: contact model and application to cemented sand, *Computers and Geotechnics* 75 (2016) 192–209.
- [38] R. Affes, J. Y. Delenne, Y. Monerie, F. Radjai, V. Topin, Tensile strength and fracture of cemented granular aggregates, *European Physical Journal E* 35 (11) (2012). doi:10.1140/epje/i2012-12117-7.
- [39] V. Langlois, X. Jia, Acoustic probing of elastic behavior and damage in weakly cemented granular media, *Physical Review E* 89 (2014) 023206.
- [40] J. Dvorkin, H. Yin, Contact laws for cemented grains: implications for grain and cement failure, *International Journal of Solids and Structures* 32 (17/ 18) (1995) 2497–2510.
- [41] V.-D. Than, S. Khamseh, A. M. Tang, J.-M. Pereira, F. Chevoir, J.-N. Roux, Basic Mechanical Properties of Wet Granular Materials: A DEM Study, *ASCE Journal of Engineering Mechanics* (2016).

- [42] G. Jefferson, G. K. Haritos, R. M. McMeeking, The elastic response of a cohesive aggregate—a discrete element model with coupled particle interaction, *Journal of the Mechanics and Physics of Solids* 50 (12) (2002) 2539–2575.
- [43] I. Agnolin, J.-N. Roux, Internal states of model isotropic granular packings. II. Compression and pressure cycles, *Phys. Rev. E* 76 (6) (2007) 061303.
- [44] J.-N. Roux, F. Chevoir, Discrete numerical simulation and the mechanical behavior of granular materials, *Bulletin des Laboratoires des Ponts et Chaussées* 254 (2005) 109–138.
- [45] J.-N. Roux, F. Chevoir, Dimensional Analysis and Control Parameters, in: Radjaï and Dubois [17], Ch. 8, pp. 199–232.
- [46] E. Somfai, M. van Hecke, W. G. Ellenbroek, K. Shundyak, W. van Saarloos, Critical and noncritical jamming of frictional grains, *Phys. Rev. E* 75 (2) (2007) 020301.
- [47] M. Wyart, On the rigidity of amorphous solids, *Annales de Physique Fr.* 30 (2006) 1–96.
- [48] P. J. Digby, The effective elastic moduli of porous granular rocks, *Journal of Applied Mechanics* 48 (4) (1981) 803.
- [49] R. Kumar, S. Rommel, D. Jauffrès, P. Lhuissier, C. L. Martin, Effect of packing characteristics on the discrete element simulation of elasticity and buckling, *International Journal of Mechanical Sciences* 110 (2016) 14–21.
- [50] P.-E. Peyneau, J.-N. Roux, Solidlike behavior and anisotropy in rigid frictionless bead assemblies, *Phys. Rev. E* 78 (2008) 041307.
- [51] F. Radjai, Modeling force transmission in granular materials, *Comptes Rendus Physique* 16 (1) (2015) 3–9.
- [52] M. H. Khalili, J.-N. Roux, J.-M. Pereira, S. Brisard, M. Bornert, A numerical study of one-dimensional compression of granular materials: I. Stress-strain behavior, microstructure and irreversibility, *Phys. Rev. E* 95 (032907) (2017).

- [53] L. La Ragione, V. Magnanimo, Contact anisotropy and coordination number for a granular assembly: A comparison of distinct-element-method simulations and theory, *Physical Review E* (2012) 031304.
- [54] T. Aste, M. Saadatfar, T. J. Senden, The geometrical structure of disordered sphere packings, *Phys. Rev. E* 71 (2005) 061302.
- [55] S. Herminghaus, Dynamics of wet granular matter, *Advances in Physics* 54 (2005) 3, 221–261.
- [56] R. Mani, D. Kadau, D. Or, H. J. Herrmann, Fluid depletion in shear bands, *Phys. Rev. Lett.* 109 (2012) 248001:1–5.
- [57] M. Badetti, A. Fall, D. Hautemayou, F. Chevoir, P. Aimedieu, S. Rodts, J. N. Roux, Rheology and microstructure of unsaturated wet granular materials: Experiments and simulations, *Journal of Rheology* 62 (5) (2018) 1175–1186.